

Supplementary information

Verification of seismic enforced-displacement pushover procedure on torsionally flexible, asymmetric, multi-storey r/c buildings

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Figure S1 shows the numbering of the structural members in the nonlinear model of the six -storey r/c building. The dimensions of the cross-sections and the reinforcement details are presented in Tables A1-A3.

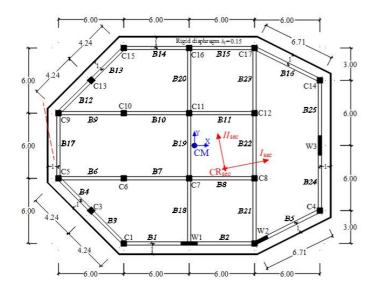


Figure S1. Numbering of the structural members in the nonlinear model of the six-storey r/c building.

Table S1. Geometry of cross sections.

Structural alament	Storey								
Structural element	1	2	3	4	5	6			
All square columns	50/50								
All T-beams	T 30/60/150/15								
Wall W1	150/30	150/30							
Wall W2	150/30/50/50								
7all W3 30/180									

Table S2. Reinforcement details of T beams' end-sections (closed perimeter stirrup Ø8 mm placed every 80 mm).

	Storey											
	1		2		3		4		5		6	
Beam	start	end	start	end	start	end	start	end	start	end	start	end
	Longit	udinal	bars Ø	16 mm	or Ø1	4 mm	ı (*) at beam e		end-sections, placed on top-			
	bottom fibers											
B 1	6-5	8-6	7-5	9-7	7-5	9-7	5-4	7-5	5-4 *	5-4	4-4 *	4-4 *
2	8-6	9-7	9-7	10-8	9-7	9-7	7-5	8-6	5-4	6-5	4-4 *	4-4
3	4-4	4-4	5-5	5-5	5-5	5-5	4-4	4-4	4-4 *	4-4 *	4-4 *	4-4 *
4	4-4	4-4	5-5	5-5	5-5	5-5	4-4	4-4	4-4 *	4-4 *	4-4 *	4-4 *
5	6-5	6-5	8-6	7-6	8-6	7-6	6-5	5-5	4-4	4-4	4-4 *	4-4 *
6	5-4	5-4	5-4	6-5	5-4	6-5	4-4	4-4	4-4 *	4-4	4-4 *	4-4 *
7	5-4	5-4	6-5	6-5	6-5	6-5	4-4	4-4	4-4	4-4 *	4-4 *	4-4 *
8	5-4	5-4	6-5	6-5	6-5	6-5	4-4	4-4	4-4 *	4-4 *	4-4 *	4-4 *
9	5-4	6-4	6-4	6-4	5-4	6-4	4-4	5-4	4-4 *	4-4	4-4 *	4-4 *
10	6-4	6-4	6-4	6-4	6-4	5-4	5-4	4-4	4-4	4-4 *	4-4 *	4-4 *

11	6-4	6-5	6-4	7-5	5-4	6-5	4-4	5-4	4-4 *	4-4 *	4-4 *	4-4 *
12	5-5	5-4	5-5	6-4	4-4	5-4	4-4	4-4	4-4 *	4-4 *	4-4 *	4-4 *
13	5-4	6-5	6-4	6-5	5-4	5-4	4-4	4-4	4-4 *	4-4 *	4-4 *	4-4 *
14	7-5	6-5	7-5	7-5	7-5	6-5	5-4	5-4	4-4	4-4 *	4-4 *	4-4 *
15	6-5	6-5	7-5	7-5	6-5	6-5	5-4	5-4	4-4 *	4-4 *	4-4 *	4-4 *
16	7-5	8-6	8-6	8-6	7-5	7-5	6-5	6-5	4-4	4-4	4-4 *	4-4 *
17	4-4	4-4	5-4	5-4	5-4	5-4	5-4	5-4	4-4	4-4	4-4 *	4-4 *
18	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4 *	4-4 *
19	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4 *	4-4 *
20	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4 *	4-4 *
21	4-4	4-4	5-4	5-4	5-4	4-4	4-4	4-4	4-4	4-4	4-4 *	4-4 *
22	4-4	4-4	5-4	5-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4 *	4-4 *
23	4-4	4-4	5-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4	4-4 *	4-4 *
24	7-6	8-6	8-6	9-7	8-6	9-7	7-6	8-6	5-4	5-4	4-4 *	4-4
25	8-6	7-5	9-7	8-6	9-7	8-6	8-6	7-6	5-4	5-4	4-4	4-4 *

^{*}Top flange reinforcement bars: 1208 mm (08/120 mm) are considered in the calculation of the bending strength of T beams.

Table S3. Reinforcement details of columns and walls.

	Storey									
(0)	1	2	3	4	5	6				
Column (C) or Wall (W)	Number of longitudinal bars and diameter (mm). Number of (closed) stirrup legs along x and y dir diameter (mm)/distance (mm) between legs									
All columns	12Ø20 confineme	ent x & y: 4 l	egs - Ø8/80		4Ø20 + 8Ø14 x & y: 4 legs - Ø8/80					
W1	(3Ø25+7Ø2 web: 10Ø1	ent x, y: 4,	web: 100		.3 m: (10Ø14) × 2 legs - Ø8/80					
W2	(16Ø20) + (web: 10Ø1	ry el. 0.5 × 3Ø25 + 7Ø20 0 ent x, y: (6, 6	0)		2 b. el. 0.5 × 0.5 m + 0.4 × 0.3 m: (12Ø20) + (10Ø14) web: 10Ø10 confinement x, y: (4, 4 legs - Ø8/80) + (4, 6 legs - Ø8/80)					
W3	(12Ø20) × web: 14Ø1	2 ent x, y: 6,	2 b. el. 0. m: (10Ø20 web: 12Ø: confinements, 4 legs -	0) × 2 12 ent x, y:	2 bound. el. 0.3 × 0.40 m; (100 × 2 web; 12012					